



The Span Tables below have been created in accordance with EN 1993-1-3 (Eurocode EC3) and calculated by the Steel Construction Institute (SCI). The values are based on a maximum permitted deflection of Span/200 under imposed load.

Load factor (working load to ultimate) 1.5 (in accordance with Eurocode).

Deflection for limit of span L/200

POSITIVE loads parameters 0.5mm

Bottom flange in compression

Moment capacity (kNm/m) 0.824

Inertia (cm⁴/m) 7.288

Bottom flange in tension

Moment capacity (kNm/m) 0.699

Inertia (cm⁴/m) 7.31

Shear resistance (kN/m) 19.953

Web crushing mid (kN/m) 4.903

Web crushing end (kN/m) 2.452

Inertia gross section (cm⁴/m) 8.596

Proclad 1000/32 - 0.5mm

Span/Load Table - GRAVITY - Working load UDL (kN/m²)

GRAVITY		Span in Metres												
Span Type	Design Case	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2
Single	Moment	3.73	3.08	2.59	2.21	1.90	1.66	1.46	1.29	1.15	1.03	0.93	0.85	0.77
	Inertia	6.24	4.69	3.61	2.84	2.27	1.85	1.52	1.27	1.07	0.91	0.78	0.67	0.59
	Reaction	3.27	2.97	2.72	2.51	2.34	2.18	2.04	1.92	1.82	1.72	1.63	1.56	1.49
	Limiting	3.27	2.97	2.59	2.21	1.90	1.66	1.46	1.27	1.07	0.91	0.78	0.67	0.59
Double	Moment	4.39	3.63	3.05	2.60	2.24	1.95	1.72	1.52	1.36	1.22	1.10	1.00	0.91
	Inertia	10.40	7.81	6.02	4.73	3.79	3.08	2.54	2.12	1.78	1.52	1.30	1.12	0.98
	Reaction	2.61	2.38	2.18	2.01	1.87	1.74	1.63	1.54	1.45	1.38	1.31	1.25	1.19
	Interaction	2.05	1.80	1.59	1.42	1.27	1.15	1.05	0.96	0.88	0.81	0.75	0.69	0.64
	Limiting	2.05	1.80	1.59	1.42	1.27	1.15	1.05	0.96	0.88	0.81	0.75	0.69	0.64
Multiple	Moment	5.49	4.54	3.81	3.25	2.80	2.44	2.15	1.90	1.70	1.52	1.37	1.25	1.13
	Inertia	10.40	7.81	6.02	4.73	3.79	3.08	2.54	2.12	1.78	1.52	1.30	1.12	0.98
	Reaction	2.97	2.70	2.48	2.29	2.12	1.98	1.86	1.75	1.65	1.56	1.49	1.42	1.35
	Interaction	2.41	2.12	1.88	1.68	1.51	1.37	1.24	1.14	1.05	0.96	0.89	0.83	0.77
	Limiting	2.41	2.12	1.88	1.68	1.51	1.37	1.24	1.14	1.05	0.96	0.89	0.83	0.77

Proclad 1000/32 - 0.5mm

Span/Load Table - UPLIFT - Working load UDL (kN/m²)

UPLIFT		Span in Metres												
Span Type	Design Case	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2
Single	Moment	4.39	3.63	3.05	2.60	2.24	1.95	1.72	1.52	1.36	1.22	1.10	1.00	0.91
	Inertia	6.23	4.68	3.60	2.84	2.27	1.85	1.52	1.27	1.07	0.91	0.78	0.67	0.58
	Reaction	3.27	2.97	2.72	2.51	2.34	2.18	2.04	1.92	1.82	1.72	1.63	1.56	1.49
	Limiting	3.27	2.97	2.72	2.51	2.24	1.85	1.52	1.27	1.07	0.91	0.78	0.67	0.58
Double	Moment	3.73	3.08	2.59	2.21	1.90	1.66	1.46	1.29	1.15	1.03	0.93	0.85	0.77
	Inertia	10.38	7.80	6.01	4.73	3.78	3.08	2.53	2.11	1.78	1.51	1.30	1.12	0.97
	Reaction	2.61	2.38	2.18	2.01	1.87	1.74	1.63	1.54	1.45	1.38	1.31	1.25	1.19
	Interaction	1.92	1.68	1.48	1.32	1.18	1.06	0.96	0.88	0.80	0.74	0.68	0.63	0.58
	Limiting	1.92	1.68	1.48	1.32	1.18	1.06	0.96	0.88	0.80	0.74	0.68	0.63	0.58
Multiple	Moment	4.66	3.85	3.24	2.76	2.38	2.07	1.82	1.61	1.44	1.29	1.17	1.06	0.96
	Inertia	10.38	7.80	6.01	4.73	3.78	3.08	2.53	2.11	1.78	1.51	1.30	1.12	0.97
	Reaction	2.97	2.70	2.48	2.29	2.12	1.98	1.86	1.75	1.65	1.56	1.49	1.42	1.35
	Interaction	2.27	1.98	1.75	1.56	1.40	1.27	1.15	1.05	0.96	0.88	0.82	0.76	0.70
	Limiting	2.27	1.98	1.75	1.56	1.40	1.27	1.15	1.05	0.96	0.88	0.82	0.76	0.70



'SCI Assessed Quality Mark'. This mark testifies that the [Steel Construction Institute \(SCI\)](#) has independently verified the technical data above.